

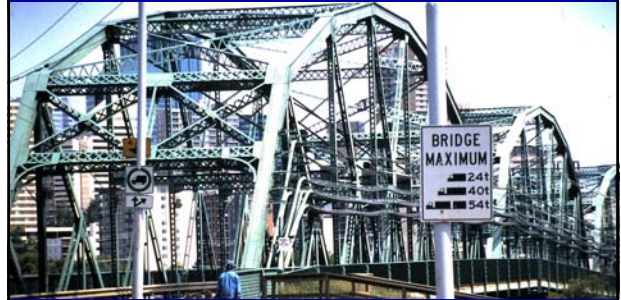
Risk-Based Evaluation of Existing Highway Bridges: Past, Present and Future

F. Michael Bartlett

Dept. of Civil & Environmental Engineering
University of Western Ontario
London, Canada

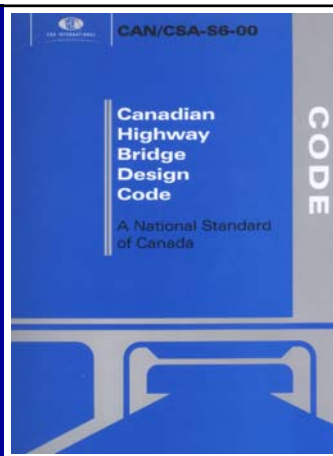
f.m.bartlett@uwo.ca

The Problem



Aging Bridge Inventory, Increasing Loads

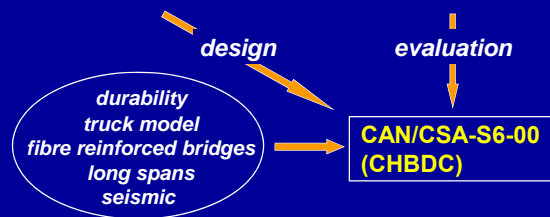
The Solution: Rational Bridge Evaluation Criteria



Criteria Unchanged Since 1990

Ontario Highway Bridge Design Code (3rd Ed., 1991)

CAN/CSA-S6, Design of Highway Bridges (8th Ed., 1988)



Presentation Objectives

- Existing Bridge Evaluation
 - Constant Risk Basis
- New Directions
 - Quantify Warning

Constant Risk – Life Safety

Risk = probability X consequences

$$P_f W \sqrt{n} = A K$$

Probability of failure

Consequences:

- W – warning
- n – number of people at risk

Constant

Factors that Provide Warning

- System Behaviour
 - S1 (Single Load Path), to
 - S3 (Multiple Load Path)
- Element Behaviour
 - E1 (Brittle), to
 - E3 (Ductile)
- Inspection Classification
 - INSP1 (Uninspectable), to
 - INSP 3 (Evaluator inspects deficient members)

Impact: Concrete Arch Bridge

- Consider
- Top Chord
 - Floor Beam

Umpqua River Bridge,
Reedsport, Oregon



Top Chord (Compression)

- System: S1
 - single load path
- Element Behav.: E1
 - Brittle
- Inspection: INSP2
 - Routine
- Target β : 3.75



Floor Beam (Flexure)

- System: S2
 - Not SLP
- Element Behav.: E3
 - Ductile
- Inspection: INSP2
 - Routine
- Target β : 3.00



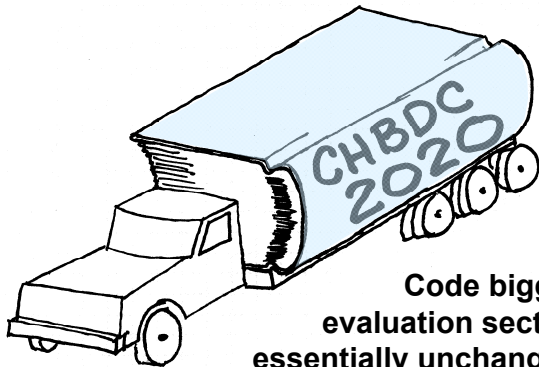
Impact on Rating

	Chord	Beam
Target β	3.75	3.00
Concrete DL	1.2 x 500	1.14 x 50
Asphalt DL	1.5 x 50	1.35 x 5
LL (incl DLA)	1.7 x 600	1.49 x 60
Factored Demand	1695	153
Factored Capacity	1550	155
Result	insufficient	ok

Economic Impact

- Short span elements including bridge floor systems susceptible to increased traffic loads.
- These elements tend to be ductile, readily inspected, and part of a multiple load path.
- Less stringent β allows them to be deemed adequate.
- Marked economic savings achieved.

Present: 2020 Design Truck?



Future: Quantify Warning

1. Deflection as a metric of warning
2. Computing deflection at imminent failure, Δ_t :
 - Cross section response
 - Application of moment-area method
3. Computing warning factor, W , given Δ_t/L

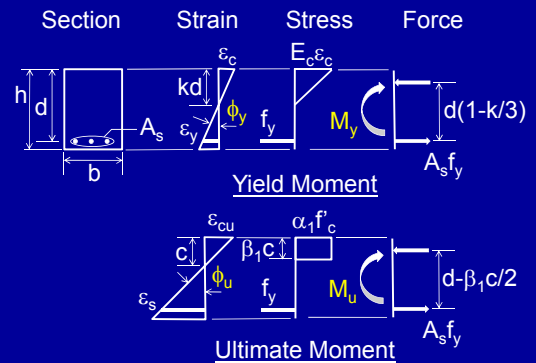
Deflection = Warning



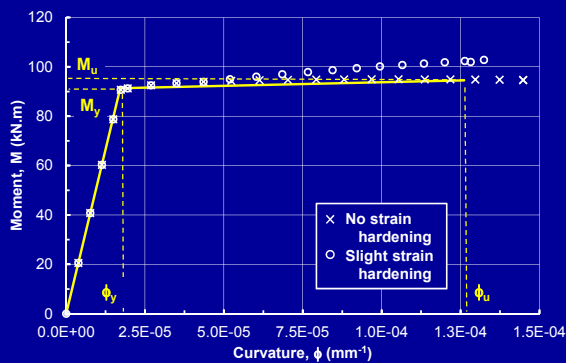
Bridge over Schubencadie River near Truro, NS



Conventional Flexural Analysis



Bilinear Idealization



Shape Factor

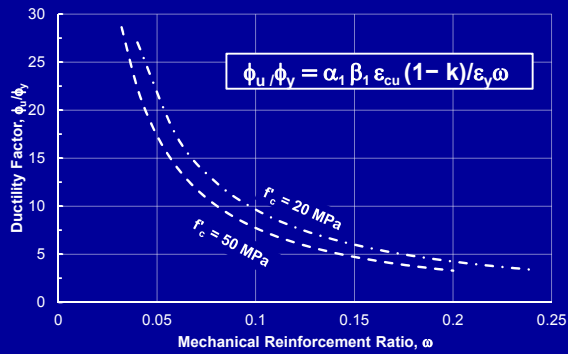
- Shape Factor $f = M_u/M_y$

$$f = (1 - \omega/2\alpha_1) / (1 - k/3)$$

where ω , the mechanical reinforcing ratio, $= A_s f_y / (b d f'_c)$

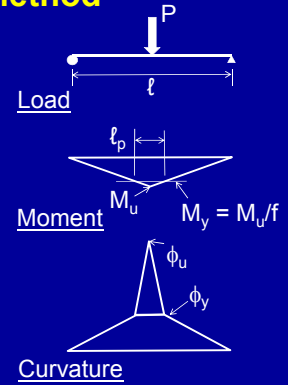
- Typically $1.01 < f < 1.05$ for $\omega < 0.3$

Curvature Ductility Factor, ϕ_u/ϕ_y



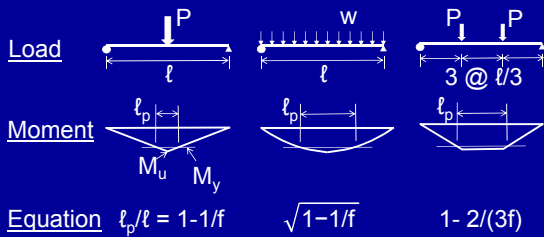
Moment Area Method

- Integrate curvature diagram accounting for plastic hinging
- Get Δ_i : deflection at incipient failure

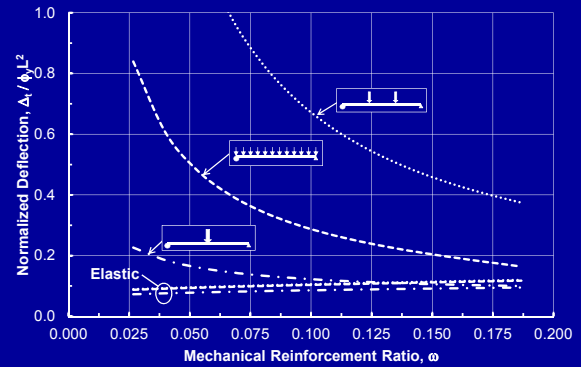


Plastic Hinge Length Varies

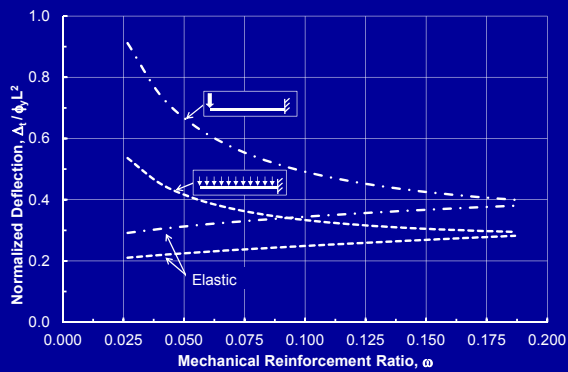
- Function of shear force in plastic hinge region – and Shape Factor f



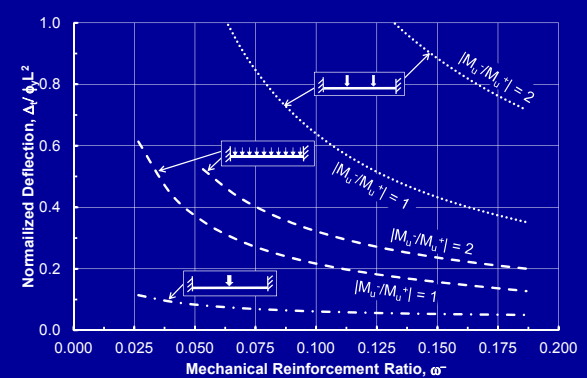
Simply Supported Beams



Cantilevers



Fixed-ended Beams



Express Δ_t as Δ_t/L

- Common practical measure
- Can show $\Delta_t/L = \alpha \Delta_t/(\phi_y L^2)$

$$\text{where } \alpha = \frac{\epsilon_y}{(1-k)} \left(\frac{h}{d} \right) \frac{L}{h}$$

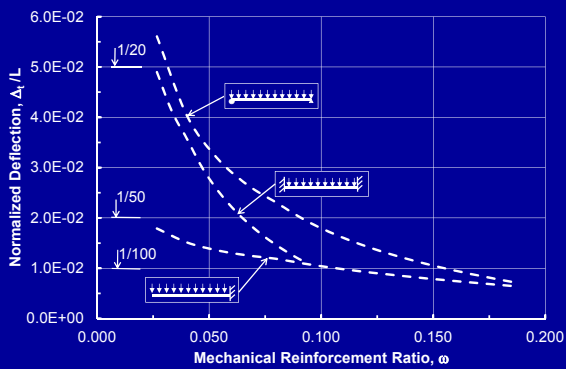
say $(h/d) \sim 1/0.85$ for a thin slab to
 $\sim 1/0.95$ for a deep beam

(L/h) from Deflection Control Limits

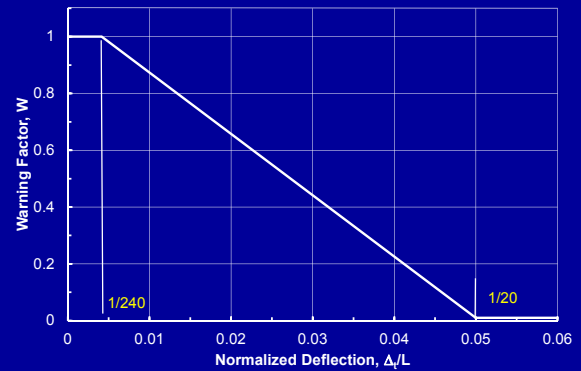
- ACI 318 & A23.3 provide maximum L/h limits that, if satisfied, do not require deflections to be checked.

Beam end restraints	Span-to-depth ratio, L/h		Avg. α
	1-way Slab	Beam	
Simple support	20	16	0.054
Cantilever	10	8	0.027
Fixed ends	28	21	0.073

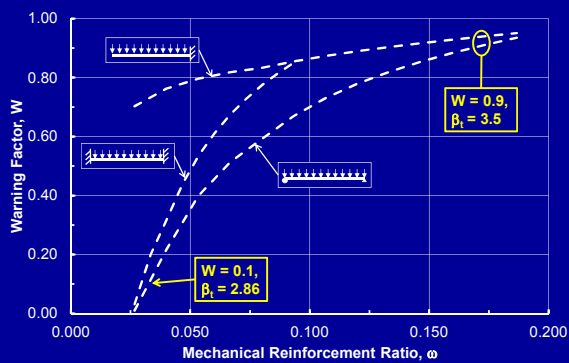
Beams with UDLs



Relate W to Δ_t/L



W Variation with ω



Conclusions

1. Constant risk criteria in CHBDC require more safety for members with severe consequences of failure.
2. Bridge members that are sensitive to higher modern traffic loadings get a break.
3. Significant economies are achieved.

4. Deflection at imminent failure captures warning of failure.
5. Increased deflections for:
 - Ductile cross sections
 - Long plastic hinge lengths (load configurations matter!)
6. Redundancy is an inconsistent indicator of warning of failure.
7. Can rationally quantify warning factor as a continuous variable.

Acknowledgements

- Natural Sciences and Engineering Research Council of Canada
- American Concrete Institute
- Japan Concrete Institute
- Various academic and professional colleagues

